

ICC-ES Evaluation Report

ESR-1752

Reissued September 2023 This report also contains:

Revised March 2024 - LABC Supplement

Subject to renewal September 2025 - FBC Supplement

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DIVISION: 03 00 00-

CONCRETE

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MASONRY

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METALS

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Fastenings

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FINISHES

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Fasteners

REPORT HOLDER:

HILTI, INC.

EVALUATION SUBJECT: HILTI LOW-VELOCITY POWER-ACTUATED

FASTENERS



1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2021, 2018 and 2015 International Building Code® (IBC)
- 2021, 2018 and 2015 International Residential Code® (IRC)

For evaluation for compliance with codes adopted by the <u>Los Angeles Department of Building and Safety</u> (LADBS), see ESR-1752 LABC and LARC Supplement.

Property evaluated:

■ Structural

2.0 USES

Hilti low-velocity power-actuated fasteners are used with powder-actuated, gas-driven or electro-mechanically driven tools to attach light-gage cold-formed steel framing and nonstructural components to base materials of normalweight concrete, sand-lightweight concrete, metal deck panels with sand-lightweight concrete fill, concrete masonry and structural steel. The fasteners are alternatives to the cast-in-place anchors described in IBC Section 1901.3 for placement in concrete; the embedded anchors described in Section 8.1.3 of TMS 402, referenced in Section 2107 of the IBC for placement in masonry; and the welds and bolts used to attach materials to steel, addressed in IBC Sections 2204.1 and 2204.2. For structures regulated under the IRC, the fasteners may be used where an engineered design is submitted in accordance with IRC Section R301.1.3.

3.0 DESCRIPTION

3.1 Fasteners:

Hilti low-velocity power-actuated fasteners (PAFs) are manufactured from hardened steel complying with the manufacturer's quality documentation. See <u>Table 1</u> for shank type, fastener dimensions and coating description. Maximum point length is the maximum specified length from the tip of the fastener to the location where the diameter of the shank becomes constant. Minimum effective shank length is the minimum specified length from the underside of the fastener head to the tip of the fastener.

3.2 Substrate Materials:

- **3.2.1 Concrete:** Normalweight and sand-lightweight concrete must comply with IBC Chapter 19 or IRC Section R402.2, as applicable. The minimum concrete compressive strength at the time of fastener installation must be as noted in the applicable allowable load table.
- **3.2.2 Concrete Masonry:** Concrete masonry units (CMUs) must be minimum 8-inch-thick (203 mm), and must comply with ASTM C90. Mortar must comply with ASTM C270. Grout must be course grout complying with ASTM C476. Concrete masonry walls must have minimum compressive strength, f'_m , of 1,500 psi (10.3 MPa). See Table 6 for applicable CMU density and mortar type.
- **3.2.3 Steel:** Structural steel supports must comply with the minimum requirements of ASTM A36, ASTM A572 Grade 50 or ASTM A992, as applicable, and must have minimum yield and tensile strengths and thickness as noted in <u>Table 2</u>.
- **3.3 Steel Deck Panels:** Steel deck panel properties and configurations must be as described in footnote 4 of <u>Tables 4</u> and <u>5</u> and <u>Figures 16</u> through <u>18</u>.

4.0 DESIGN AND INSTALLATION

4.1 Design:

- **4.1.1 General:** Selection of fasteners must take into consideration the applicable base material and the length of the fastener. The minimum fastener length must be determined as follows:
- For installation into concrete, concrete-filled steel deck panels, concrete masonry and steel base materials, the minimum effective shank length shown in <u>Table 1</u> must equal or exceed the sum of the thickness of the attached material and the minimum embedment depth (penetration) shown in the applicable tables in this report.
- For installation through steel base materials, the minimum effective shank length shown in <u>Table 1</u> must equal or exceed the sum of the following: the thickness of the attached material, the thickness of the base material and the required point penetration shown in the applicable tables in this report.
- **4.1.2** Allowable Loads: The applicable allowable load tables for Hilti powder-actuated fasteners driven into different base materials may be determined by referencing <u>Table 1</u>.

The most critical applied loads, excluding seismic load effects, resulting from the load combinations in Section 2.4 of ASCE 7-16/S1 (referenced in 2021 IBC Section 1605.1) or 2021 IBC Section 1605.2 (Section 1605.3.1 or 1605.3.2 of the 2018 and 2015 IBC) must not exceed the allowable loads. For fasteners which are subjected to seismic loads, see Section 4.1.5 for additional information. The stress increases and load reductions described in 2021 IBC Section 1605.2 (2018 and 2015 IBC Section 1605.3) are not allowed.

Allowable shear loads and tension (pullout) loads in this report apply to the connection of the fastener to the base material. Other limit states applicable to the design of a connection, such as fastener pull-through (pullover) and lateral bearing on the attached material, which are governed by the properties of attached materials, are outside the scope of this report. Design of the connection to the attached material must comply with the applicable requirements of the IBC.

4.1.3 Combined Loading: For fasteners subjected to both tension and shear loads, compliance with the following interaction equation must be verified:

 $(p/P_a) + (v/V_a) \le 1$

where:

p = Actual applied tension load on fastener, lbf (N).

 P_a = Allowable tension load on fastener, lbf (N).

v = Actual applied shear load on fastener, lbf (N).

 V_a = Allowable shear load on fastener, lbf (N).

- **4.1.4 Steel-to-steel Connections:** When the Hilti fasteners listed in <u>Table 2</u> are used in connections of two steel elements in accordance with Section J5 of AISI S100 (Section E5 of AISI S100-12 for the 2015 IBC), connection capacity must be determined in accordance with Sections 4.1.4.1 and 4.1.4.2, as applicable.
- **4.1.4.1 Connection Strength—Tension:** To determine tensile connection strength in accordance with Section J5.2 of AISI S100 (Section E5.2 of AISI S100-12), fastener tension strength, the pull-out strength and the pull-over strength must be known. These characteristics must be determined as follows:
- Pull-out Strength: See <u>Table 2</u> for available pull-out strength.
- Pull-over Strength: The available pull-over strengths must be calculated in accordance with Section J5.2.3 of AISI S100 (Section E5.2.3 of AISI S100-12).
- PAF Tensile Strength: The allowable fastener tension strengths, determined in accordance with Section J5.2.1 of AISI S100 (Section E5.2.1 of AISI S100-12), exceed the allowable pull-out strengths in Table 2.
- **4.1.4.2 Connection Strength—Shear:** To determine shear connection strength in accordance with Section J5.3 of AISI S100 (Section E5.3 of AISI S100-12), the fastener shear strength, bearing and tilting strength, pull-out strength in shear, net section rupture strength and shear strength limited by edge distance must be known. These characteristics must be determined as follows:
- Bearing and Tilting Strength: The available bearing and tilting strengths must be calculated in accordance with Section J5.3.2 of AISI S100 (Section E5.3.2 of AISI S100-12).
- Pull-out Strength in Shear: The available pull-out strength in shear must be the applicable allowable shear strength from <u>Table 2</u> or must be calculated in accordance with Section J5.3.3 of AISI S100 (Section E5.3.3 of AISI S100-12).
- Net Section Rupture Strength and Shear Strength Limited by Edge Distance: The net section rupture strength must be determined in accordance with Section J5.3.4 of AISI S100 (Section E5.3.4 of AISI S100-12) and the shear strength limited by edge distance must be determined in accordance with Section J5.3.5 of AISI S100 (Section E5.3.5 of AISI S100-12).
- PAF Shear Strength: The allowable fastener shear strengths, determined in accordance with Section J5.3.1 of AISI S100 (Section E5.3.1 of AISI S100-12), exceed the allowable pull-out in shear strengths in <u>Table 2</u>.
- 4.1.5 Seismic Considerations: When subjected to seismic loads, the Hilti fasteners may be used as follows:
- 1. The Hilti fasteners may be used for attachment of nonstructural components listed in Section 13.1.4 of ASCE/SEI 7, which are exempt from the requirements of ASCE/SEI 7.
- 2. Concrete base materials: The Hilti fasteners installed in concrete may be used to support distributed systems and distribution systems where the service load on any individual fastener does not exceed the lesser of 90 lbf (400 N) or the allowable load shown in <u>Tables 3</u>, <u>4</u> and <u>5</u>, as applicable.
- 3. Steel base materials: When the Hilti fasteners listed in <u>Table 2</u> (except for the X-S 14 G2) are installed in steel base materials and subjected to seismic load, the most critical load applied to each individual fastener must be determined from the equations in Section 2.4 of ASCE 7-16/S1 (referenced in 2021 IBC Section 1605.1) or 2021 IBC Section 1605.2 (2018 and 2015 IBC Section 1605.3.1 or Section 1605.3.2) which include seismic load effects, and must not exceed the allowable load shown in <u>Table 2</u>. The X-P G2 fasteners may be used for attaching nonstructural components where the service load on any individual fastener does not exceed the lesser of 250 lbf (1112 N) or the published allowable load shown in <u>Table 2</u>.
- 4. For interior, nonstructural walls that are not subject to sustained tension loads and are not a bracing application, the power-actuated fasteners may be used to attach steel track to concrete or steel in all Seismic Design Categories. In Seismic Design Categories D, E, and F, the allowable shear load due to transverse pressure must be no more than 90 pounds (400 N) when attaching to concrete; or the allowable load described in Item 3, above, when attaching to steel. Substantiating calculations must be submitted addressing the fastener-to-base-material capacity and the fastener-to-attached-material capacity. Interior nonstructural walls are limited to locations where bearing walls, shear walls or braced walls are not required by the approved plans. The design load on the fastener must not exceed the allowable load established in this report for the concrete or steel base material.

4.2 Installation:

The fasteners must be installed in accordance with this report and the Hilti, Inc. published installation instructions. A copy of these instructions must be available on the jobsite at all times during fastener installation.

Fastener installation requires the use of a low-velocity power-actuated tool (gas, powder or electro-mechanical actuated) in accordance with Hilti, Inc. recommendations. Installers of powder-actuated fasteners must be certified by Hilti, Inc., and have a current, Hilti-issued, operator's license. Installers of gas-driven or electro-mechanical-driven fasteners do not require an operator's license.

The fastener size, minimum embedment depth or penetration, minimum spacing, and edge distances must comply with <u>Tables 2</u> through <u>6</u>, as applicable. For fasteners installed into concrete or masonry, the fasteners must not be driven until the concrete or masonry has reached the designated compressive strength.

5.0 CONDITIONS OF USE:

The Hilti low-velocity power-actuated fasteners described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 Fasteners must be manufactured and identified in accordance with this report.
- **5.2** Fasteners must be installed in accordance with this report and the Hilti, Inc. instructions. In the event of conflict between this report and the Hilti, Inc. published instructions, the more restrictive requirements govern.
- 5.3 Calculations demonstrating that the applied loads are less than the allowable loads described in this report must be submitted to the code official for approval. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is constructed.
- 5.4 For steel-to-steel connections that meet the applicability requirements of Section J5 of AISI S100 (Section E5 of AISI S100-12 for the 2015 IBC), calculations demonstrating that the available connection strength has been determined in accordance with Section J5 of AISI S100 (Section E5 of AISI S100-12) and Section 4.1.4 of this report, and equals or exceeds the applied load, must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
- **5.5** Refer to Section 4.1.5 for seismic considerations.
- **5.6** The fasteners must be limited to dry, interior locations, which include exterior walls which are protected by an exterior wall envelope.
- **5.7** The use of fasteners in concrete or masonry is limited to installation in uncracked concrete or masonry. Cracking occurs when $f_t > f_r$ due to service loads or deformations.
- **5.8** The Hilti products addressed in this report are manufactured under a quality-control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Power-actuated Fasteners Driven into Concrete, Steel, and Masonry Elements (AC70), dated December 2019 (editorially revised January 2021), including seismic load test data in accordance with Annex A of AC70.

7.0 IDENTIFICATION

- **7.1** The ICC-ES mark of conformity, electronic labeling, or the evaluation report number (ICC-ES ESR-1752) along with the name, registered trademark, or registered logo of the report holder must be included in the product label.
- 7.2 In addition, the Hilti low-velocity power-actuated fasteners, collated into plastic strips of ten, are identified by an "H" imprinted on the fastener head. All fasteners are packaged in containers that bear the fastener type and size, the report holder's name (Hilti, Inc.), and the evaluation report number (ESR-1752).
- **7.3** The report holder contact information is the following:

HILTI, INC.
7250 DALLAS PARKWAY, SUITE 1000
PLANO, TEXAS 75024
(800) 879-8000
www.hilti.com
HNATechnicalServices@hilti.com

TABLE 1—FASTENER DESCRIPTION AND APPLICATIONS¹

FASTENER	FASTENER DESCRIPTION	SHANK TYPE	SHANK DIAMETER [inch (mm)]	HEAD DIAMETER [inch (mm)]	MAXIMUM POINT LENGTH [inch (mm)]	MINIMUM EFFECTIVE SHANK LENGTH ² [inch (mm)]	FASTENER COATING	APPLICABLE BASE MATERIAL	APPLICABLE LOAD TABLES
X-C ##	Powder Actuated Standard Fastener (<u>Figure 1</u>)	Knurled, straight	0.138 (3.5)	0.321 (8.15)	0.295 (7.5)	See Footnote 2	ASTM B633, SC1, Type III	Concrete Concfilled deck CMU	3 ,4, 5, 6
X-C22 P8TH	Powder Actuated Standard Fastener w/ Metal Tophat Washer (Figure 2)	Knurled, straight	0.138 (3.5)	0.321 (8.15)	0.295 (7.5)	0.807 (20.5)	ASTM B633, SC1, Type III	Concrete Concfilled deck	3, 4, 5
X-C20 THP	Powder Actuated Standard Fastener w/ Plastic Tophat Washer (Figure 3)	Knurled, straight	0.138 (3.5)	0.321 (8.15)	0.295 (7.5)	0.728 (18.5)	ASTM B633, SC1, Type III	Concrete Concfilled deck	4
X-S13 THP	Powder Actuated Steel Fastener w/ Plastic Tophat Washer (Figure 4)	Knurled, straight	0.145 (3.7)	0.321 (8.15)	0.295 (7.5)	0.461 (11.7)	ASTM B633, SC1, Type III	Steel	2
X-S16P8TH	Powder Actuated Steel Fastener w/ Metal Tophat Washer (Figure 2)	Smooth, tapered	0.145 (3.7)	0.323 (8.2)	0.362 (9.2)	0.571 (14.5)	ASTM B633, SC1, Type III	Steel	2
X-C 39 G2 X-C 39 G3	Standard Gas Driven Fastener (Figure 5)	Knurled, straight	0.101 (2.6)	0.228 (5.8)	0.224 (5.7)	1.476 (37.5)	ASTM B633, SC1, Type III	Concrete CMU	3, 6
X-C 20 G3	Standard Gas Driven					0.709		Concrete	
X-C ## G3 (except for X- C 39 G3)	Fastener (<u>Figure 6</u>)	Knurled, straight	0.118 (3.0)	0.256 (6.5)	0.209 (5.3)	See Footnote 2	ASTM B633, SC1, Type III	Concrete Concfilled deck CMU	3, 4, 5, 6
X-S 14 G3	Premium Gas Driven Fastener (Figure 7)	Smooth, tapered	0.118 (3.0)	0.268 (6.8)	0.394 (10.0)	0.512 (13.0)	2-to-10-micron Zinc	Steel	2
X-P ## G3	Premium Gas Driven Fastener (Figure 7)	Smooth, tapered	0.118 (3.0)	0.268 (6.8)	0.394 (10.0)	See Footnote	2-to-10-micron Zinc	Steel Concrete Concfilled deck	2, 3, 4, 5
X-C 20 G2 X-C 27 G2 X-C 32 G2	Standard Gas Driven Fastener (Figure 8)	Knurled, straight	0.108 (2.75)	0.248 (6.3)	0.193 (4.9)	0.711 (18.1) 1.039 (26.4) 1.211 (30.8)	2-to-10-micron Zinc	Concrete Concfilled deck CMU	3, 4, 5, 6
X-S 14 G2	Premium Gas Driven Fastener (Figure 9)	Knurled, tapered	0.118 (3.0)	0.248 (6.3)	0.189 (4.8)	0.512 (13.0)	8-to-16-micron Zinc	Steel	2
X-P 17 G2	Premium Gas Driven					0.630 (16.0)		Steel	
X-P 20 G2	Fastener (<u>Figure 9</u>)	Smooth, tapered	0.118 (3.0)	0.260 (6.6)	0.394 (10.0)	0.748 (19.0)	2-to-10-micron Zinc	Concrete Concfilled deck	2, 3, 4, 5
X-C 20 B3 X-C ## B3 (except for X- C 36 B3)	Electro-mechanical Driven Fastener (<u>Figure 10</u>)	Knurled, straight	0.118 (3.0)	0.256 (6.5)	0.209 (5.3)	0.709 (18.0) See Footnote 2	ASTM B633, SC1, Type III	Concrete Concfilled deck CMU	3, 4, 5, 6
X-C 36 B3	Electro-mechanical Driven Fastener (Figure 10)	Knurled, straight	0.108 (2.75)	0.248 (6.3)	0.193 (4.9)	1.378 (35.0)	2-to-10-micron Zinc	Concrete Concfilled deck CMU	3, 4, 5, 6
X-S 14 B3	Electro-mechanical Driven Fastener (Figure 11)	Smooth, tapered	0.118 (3.0)	0.268 (6.8)	0.394 (10.0)	0.512 (13.0)	2-to-10-micron Zinc	Steel	2
X-P ## B3	Electro-mechanical Driven Fastener (Figure 11)	Smooth, tapered	0.118 (3.0)	0.268 (6.8)	0.394 (10.0)	See Footnote	2-to-10-micron Zinc	Steel Concrete Concfilled deck	2, 3, 4, 5
X-C 20 B4	Electro-mechanical					0.709 (18.0)		Concrete	
X-C ## B4 (except for X- C 39 B4)	Driven Fastener (Figure 12)	Knurled, straight	0.118 (3.0)	0.256 (6.5)	0.209 (5.3)	See Footnote 2	ASTM B633, SC1, Type III	Concfilled deck CMU	3, 4, 5, 6
X-C 39 B4	Electro-mechanical Driven Fastener (<u>Figure 13</u>)	Knurled, straight	0.108 (2.75)	0.248 (6.3)	0.193 (4.9)	1.476 (37.5)	2-to-10-micron Zinc	Concrete Concfilled deck CMU	3, 4, 5, 6
X-S 14 B4	Electro-mechanical Driven Fastener (<u>Figure 14</u>)	Smooth, tapered	0.118 (3.0)	0.268 (6.8)	0.394 (10.0)	0.512 (13.0)	2-to-10-micron Zinc	Steel	2
X-P ## B4	Electro-mechanical Driven Fastener (<u>Figure 15</u>)	Smooth, tapered	0.118 (3.0)	0.268 (6.8)	0.394 (10.0)	See Footnote 2	2-to-10-micron Zinc	Steel Concrete Concfilled deck	2, 3, 4, 5

For **SI:** 1 inch = 25.4 mm.

¹## denotes numbers used in fastener designation to represent nominal fastener length in mm.

²When multiple lengths of a fastener are addressed, the minimum effective shank length can be calculated in terms of the designated length as (##-1) in mm and (##-1)/25.4 in inches.

TABLE 2—ALLOWABLE LOADS FOR FASTENERS DRIVEN INTO STEEL^{1,2,3,4}

FASTENER	SHANK DIAMETER (INCH)	ALLOWABLE LOADS (lbf)												
Steel Thick	Steel Thickness (inch):		1/8		³ / ₁₆		1/4		3/8		1/2		3/4	
Load D	irection:	Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear	Tension	Shear	
X-S13 THP	0.145	140 ¹⁰	300	300 ¹⁰	450	300 ¹⁰	450	300 ¹⁰	450					
X-S16P8TH	0.145			225 ¹⁰	420	225 ¹⁰	430	225 ¹⁰	430	225 ¹⁰	430			
X-S 14 B3 X-S 14 B4 X-S 14 G3	0.118	140	230	220	245	225	290	280 ⁶	330 ⁶	280 ⁶	330 ⁶	280 ⁶	330 ⁶	
X-S 14 B3 ⁵ X-S 14 B4 ⁵ X-S 14 G3 ⁵	0.118			220	295	260	355	280 ⁶	385 ⁶	280 ⁶	385 ⁶	280 ⁶	385 ⁶	
X-P ## G3 X-P ## B3 X-P ## B4	0.118	125 ¹⁰	230	170 ¹⁰	245	20010	230	250 ¹⁰	255					
X-P 17 G2 ⁷ X-P 20 G2 ⁷	0.118			140 ¹⁰	220	180 ⁸	2008	225 ⁶	220 ⁶					
X-S 14 G2 ⁷	0.118		-		-	215 ⁸	290 ⁸	150 ⁹	195 ⁹	130 ⁹	150°	130 ⁹	150 ⁹	

For **SI**: 1 inch = 25.4 mm, 1 ksi = 6.89 MPa, 1 lbf = 4.45 N.

Unless otherwise noted, fasteners must be driven to where the full length of the point of the fastener penetrates through the steel base material.

 $^{^{2}}$ Unless otherwise noted, steel base material must have minimum yield and tensile strengths (F_{y} and F_{u}) equal to 36 ksi and 58 ksi, respectively.

³Unless otherwise noted, allowable loads are applicable to static loads and seismic loads in accordance with Section 4.1.

⁴Fastener spacing must be a minimum of 1.0 inch and edge distance must be a minimum of 0.50 inch.

⁵Steel base material must have minimum yield and tensile strengths (F_y and F_u) equal to 50 ksi and 65 ksi, respectively.

⁶Fastener point penetration through the steel is not necessary, provided a minimum embedment of 0.320 inch is achieved.

⁷Tabulated loads for this fastener apply to static load conditions only. For seismic loading, allowable loads must be limited in accordance with Section 4.1.5, Item 3.

⁸Full fastener point penetration through the steel is not necessary, provided a minimum point penetration of 0.08 inch is achieved.

⁹Fastener point penetration through the steel is not necessary, provided a minimum embedment of 0.25 inch is achieved.

¹⁰For steel-to-steel connections designed in accordance with Section 4.1.4, the tabulated allowable load may be increased by a factor of 1.25, and the design strength may be taken as the tabulated allowable load multiplied by a factor of 2.0.

TABLE 3—ALLOWABLE LOADS FOR FASTENERS DRIVEN INTO NORMALWEIGHT CONCRETE^{1,2,3}

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FASTENER SHANK DIAMETER (inch)		MINIMUM EMBEDMENT DEPTH (inches)	ALLOWABLE LOADS (lbf)									
Concrete Cor	npressive Str	ength:	2,500) psi	4,00	0 psi	6,000 psi					
Load	Direction:		Tension	Shear	Tension	Shear	Tension	Shear				
		3/4	45	75	65	105	95	195				
X-C ##	0.138	1	85	150	160	200	105	270				
Λ-С ##	0.136	1 ¹ / ₄	130	210	270	290	165	325				
		11/2	175	260	270	4,000 psi 6,000 psi sion Shear Tension Shear 65 105 95 195 60 200 105 270 70 290 165 325 70 360 90 170 100 200 50 80 50 120 50 90 60 90						
X-C22 P8TH	0.138	3/4	55	130	90	170	100	200				
X-C 39 G2 X-C 39 G3	0.101	⁵ / ₈	50	80	50	80						
X-P 17 G2, X-P 20 G2		5/8	50	90	50	120	50	90				
X-P ## G3 X-P ## B3 X-P ## B4	0.118	3/4	80	120								
X-C ## G2 (except for X-C 39 G2) X-C 36 B3 X-C 39 B4	0.108	3/4	60	90	60	90						
X-C ## G3 (except for X-C 39 G3) X-C ## B3 (except for X-C 36 B3) X-C ## B4 (except for X-C 39 B4)	0.118	3/4	60	90	60	90						

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.45 N.

¹Fasteners must not be driven until the concrete has reached the designated minimum compressive strength, or the minimum compressive strength specified in the applicable code, whichever is greater.

²Concrete thickness must be a minimum of 3 times the embedment depth of the fastener. Fastener spacing must be a minimum of 4 inches and edge distance must be a minimum of 3 inches.

The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Section 4.1.5, as applicable. The tabulated allowable loads apply to static load conditions. For seismic load conditions, the allowable loads must be limited in accordance with Section 4.1.5, Items 2 and 4, as applicable.

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TABLE 4—ALLOWABLE LOADS FOR FASTENERS DRIVEN INTO MINIMUM 3,000 psi LIGHTWEIGHT CONCRETE AND LIGHTWEIGHT CONCRETE OVER 3-INCH-DEEP, COMPOSITE STEEL DECK PANELS 1,2,3

FASTENER	SHANK DIAMETER (inch)	MINIMUM EMBEDMENT DEPTH (inches)	ALLOWABLE LOADS (lbf)						
Factor			Fasteners	Installed	Fasteners I		rough Stee	l Deck Panel	Concrete Thickness Above Deck
Fasten	er Location:		Directly into	Concrete	Upper Lower Flute Flute		Upper Flute	Lower Flute	Panel (inches)
Load	Direction:		Tension	Shear	Tens	sion	SI		
		3/4	120	175	120	95	265	265	31/4
X-C ##	0.138	1	180	260	215	155	485	485	31/4
X-C ##		1 ¹ / ₄	225	400	250	200	500	500	31/4
		11/2	285	400	285	210	555	555	31/4
X-C22 P8TH	0.138	³ / ₄	120	220	120	95	260	260	31/4
X-C20 THP	0.138	3/4	55	110		45	285	285	31/4
X-C ## G3		³ / ₄	115	140	75	85	175	215	21/2
(except for X-C 39 G3) X-C ## B3 (except for X-C 36 B3) X-C ## B4 (except for X-C 39 B4)	0.118	1	170	220	155	160	255	315	31/4
X-P 17 G2 X-P 20 G2 X-P ## G3 X-P ## B3 X-P ## B4	0.118	5/8	60	140	60	60	175	215	21/2
X-C ## G2		3/4	110	140	75	85	175	215	21/2
(except for X-C 39 G2) X-C 36 B3 X-C 39 B4	0.108	1	170	220	155	160	255	315	3

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.45 N.

¹Fasteners must not be driven until the concrete has reached a minimum compressive strength of 3,000 psi.

²Unless otherwise noted, concrete thickness must be a minimum of 3 times the embedment depth of the fastener, fastener spacing must be a minimum of 4 inches

and edge distance must be a minimum of 3 inches.

The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Section 4.1.5, as applicable. The tabulated allowable loads apply to static load conditions. For seismic load conditions, the allowable loads must be limited in accordance with Section 4.1.5, Items 3 and 4, as applicable.

⁴⁰ ksi and a minimum tensile strength of 55 ksi. The thickness of sand-lightweight concrete fill above top of metal deck panel profiles must be as shown in the table. See Figure 16 for nominal flute dimensions, fastener locations, and load orientations.

TABLE 5—ALLOWABLE LOADS FOR FASTENERS DRIVEN INTO MINIMUM 3,000 psi SAND-LIGHTWEIGHT CONCRETE OVER $1^1/_2$ -INCH-DEEP, B-DECK STEEL PANEL 1,2,3

FASTENER	SHANK DIAMETER (inch)	MINIMUM EMBEDMENT DEPTH (inches)		Minimum Required							
Fasten	er Location:		Fasteners	Fasteners Installed Through Steel Deck Panel into Concrete ⁴							
			Upper Flute	Lower Flute	Upper Flute	Lower Flute	, ,				
Load	Direction:		Ten	sion	Sh	ear					
X-C ##	0.138	3/4	80	80 80 315		315	21/2				
X-C ##	0.130	1	205	205	445	445	21/2				
X-C22 P8TH	0.138	3/4	90	110	295	295	21/2				
X-C ## G3		3/4	75	85	175	215	21/2				
(except for X-C 39 G3) X-C ## B3 X-C ## B4 (except for X-C 39 B4)	0.118	1	155	160	255	315	31/4				
X-P 17 G2 X-P 20 G2 X-P ## G3 X-P ## B3 X-P ## B4	0.118	5/8	60	60	175	215	21/2				
X-C ## G2		3/4	75	85	175	215	21/2				
(except for X-C 39 G2) X-C 36 B3 X-C 39 B4	0.108	1	155	160	255	270	31/4				

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.89 kPa, 1 lbf = 4.45 N.

¹Fasteners must not be driven until the concrete has reached a minimum compressive strength of 3,000 psi.

²Unless otherwise noted, concrete thickness must be a minimum of 3 times the embedment depth of the fastener, fastener spacing must be a minimum of 4 inches and edge distance must be a minimum of 3 inches.

³The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Section 4.1.5, as applicable. The tabulated allowable loads apply to static load conditions. For seismic load conditions, the allowable loads must be limited in accordance with Section 4.1.5, Items 3 and 4, as applicable.

and 4, as applicable.

*Steel deck panel must be 1½-inch-deep, B-type deck panel, minimum No. 20 gage (0.0359-inch-thick base steel thickness), with a minimum yield strength of 40 ksi and a minimum tensile strength of 55 ksi. The thickness of sand-lightweight concrete fill above top of metal deck panel profiles must be as shown in the table. Fasteners may be installed through steel deck panels having either normal and inverted orientations. Fasteners must be placed at centerline of deck panel flutes. See Figures 17 and 18 for nominal flute dimensions, fastener locations, and load orientations.

TABLE 6—ALLOWABLE LOADS FOR FASTENERS DRIVEN INTO CONCRETE MASONRY 1,2,9,10

FASTENER	SHANK DIAMETER (inch)	MINIMUM EMBEDMENT DEPTH (inch)		ALLOWABLE LOADS (lbf)										
Masonry Type:					Hollow CMU Grouted CMU									
Fastener Location:					Face Shell ³		Mortar Joint		Face Shell ³		Mortar Joint		of d Cell ⁸	
Load Direction:					Shear ⁴	Tension	Shear	Tension	Shear⁴	Tension	Shear	Tension	Shear ⁴	
X-C ##	0.138	3/4	Normal weight,	40	85	20	85 ⁵	85	85	105	105⁵			
X-C ##		1	Type N minimum									180	275	
X-C ## G3	0.118	3/4	Normal weight, Type N minimum	145	190	80	80 ⁶	155	195	110	135 ⁶	105	145	
(except for X-C 39 G3) X-C ## B3 X-C ## B4 (except for X-C 39 B4)		1		185	205	105	105 ⁶	205	215	135	190 ⁶	120	150	
X-C 39 G2 X-C 39 G3	0.101	⁵ / ₈	Normal weight, Type S minimum	60	110	45	65 ⁷	85	110	55	105 ⁷			
X-C ## G2		3/4		75	140	60	80 ⁷	100	170	100	160 ⁷	80	130	
(except for X-C 39 G2) X-C 36 B3 X-C 39 B4	0.108	1	Normal weight, Type S minimum	110	190	70	145 ⁷	135	195	125	165 ⁷	110	145	

For **SI:** 1 lbf = 4.45 N, 1 inch = 25.4 mm.

¹⁰The fasteners listed in the table above may be used for static load conditions and for the seismic load conditions described in Item 1 of Section 4.1.5.



FIGURE 1—HILTI X-C ## FASTENER



FIGURE 2—HILTI X-C22 P8TH/X-S16 P8TH FASTENER







FIGURE 3—HILTI X-C20 THP FASTENER

FIGURE 4—HILTI X-S13 THP FASTENER



FIGURE 6—HILTI X-C ## G3 FASTENER (Except X-C 39 G3)

FIGURE 5—HILTI X-C 39 G3 FASTENER (X-C 39 G2 similar)



FIGURE 7—HILTI X-S 14 G3/X-P G3 FASTENER

¹See Section 3.2.2 for additional CMU, mortar and grout requirements.

²No more than one fastener may be installed in an individual masonry unit cell.

³See Figure 19 for the applicable placement zone.

⁴Shear can be applied in any direction.

⁵Applies to installation in horizontal bed joint with shear load applied parallel to the bed joint. Fastener spacing must be a minimum of 4 inches.

⁶Shear direction can be horizontal or vertical (bed joint or head joint) along the CMU wall plane.

Applies to installation in horizontal bed joint with shear load applied perpendicular to the bed joint. Fastener spacing must be a minimum of 4 inches.

⁸Fastener located in center of grouted cell, installed vertically.

⁹Fasteners must be installed a minimum of 8 inches from the end of the wall. Unless otherwise noted, multiple fasteners in a bed joint must be spaced a minimum of 8 inches.





FIGURE 8—HILTI X-C ## G2 FASTENER



FIGURE 10—HILTI X-C ## B3 FASTENER (X-C 36 B3 similar)



FIGURE 12 - HILTI X-C ## B4 FASTENER



FIGURE 14 - HILTI X-S 14 B4 FASTENER



FIGURE 9—HILTI X-S 14 G2 FASTENER (X-P ## G2 similar)



FIGURE 11—HILTI X-S 14 B3/X-P ## B3 FASTENER



FIGURE 13 - HILTI X-C 39 B4 FASTENER



FIGURE 15 - HILTI X-P ## B4 FASTENER

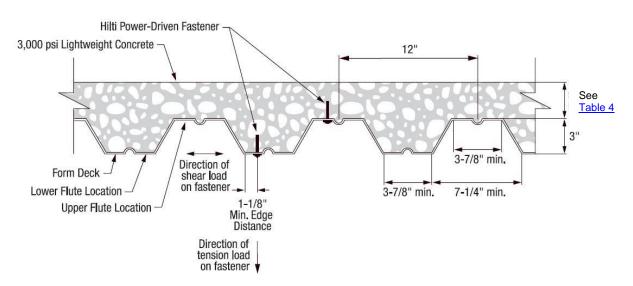


FIGURE 16—HILTI FASTENER LOCATIONS IN 3-INCH-DEEP COMPOSITE FLOOR DECK PANEL

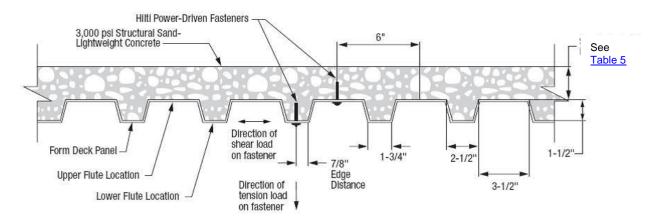
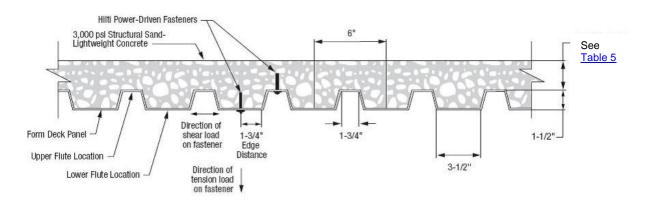


FIGURE 17—HILTI FASTENER LOCATIONS IN 11/2-INCH-DEEP COMPOSITE FLOOR DECK PANEL



For SI: 1 inch = 25.4 mm, 1 psi = 6895 Pa.

FIGURE 18—HILTI FASTENER LOCATIONS IN $1^1/_2$ -INCH-DEEP COMPOSITE FLOOR DECK PANEL, INVERTED DECK PANEL PROFILE ORIENTATION

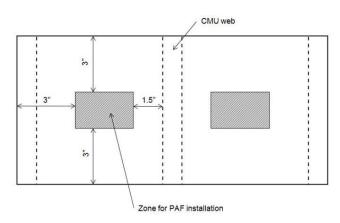


FIGURE 19—ZONE FOR FASTENER INSTALLATION IN FACE SHELL OF CMU



ICC-ES Evaluation Report

ESR-1752 LABC and LARC Supplement

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DIVISION: 05 00 00—METALS Section: 05 05 23—Metal Fastenings

DIVISION: 09 00 00—FINISHES Section: 09 22 16.23—Fasteners

REPORT HOLDER:

HILTI, INC.

EVALUATION SUBJECT:

HILTI LOW-VELOCITY POWER-ACTUATED FASTENERS

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that Hilti low-velocity power-actuated fasteners, described in ICC-ES evaluation report <u>ESR-1752</u>, have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2023 City of Los Angeles Building Code (LABC)
- 2023 City of Los Angeles Residential Code (LARC)

2.0 CONCLUSIONS

The Hilti low-velocity power-actuated fasteners, described in Sections 2.0 through 7.0 of the evaluation report ESR-1752, comply with LABC Chapters 19, 21 and 22, and the LARC, and are subjected to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The Hilti low-velocity power-actuated fasteners described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report ESR-1752.
- The design, installation, conditions of use and identification of the fasteners are in accordance with the 2021 International Building Code[®] (IBC) provisions noted in the evaluation report <u>ESR-1752</u>.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.
- The allowable load and strength design values listed in the evaluation report and tables are for the connection of the fasteners to steel, normalweight concrete, lightweight and sand-lightweight concrete over metal decks, and masonry. The connection between the fasteners and the connected members shall be checked for capacity (which may govern).

This supplement expires concurrently with the evaluation report, reissued September 2023 and revised March 2024.





ICC-ES Evaluation Report

ESR-1752 FBC Supplement

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1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the Hilti Low-velocity Power-actuated Fasteners, described in ICC-ES evaluation report ESR-1752, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2023 Florida Building Code—Building
- 2023 Florida Building Code—Residential

2.0 CONCLUSIONS

The Hilti Low-velocity Power-actuated Fasteners, described in Sections 2.0 through 7.0 of the evaluation report ESR-1752, comply with the *Florida Building Code—Building* and the *Florida Building Code—Residential*, with the replacement of the referenced ASCE 7 edition as noted below. The design requirements shall be determined in accordance with the the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable. The installation requirements noted in ICC-ES evaluation report ESR-1752 for the 2021 *International Building Code®* meet the requirements of the *Florida Building Code—Residential*, as applicable.

References to ASCE 7-16/S1 in ESR-1752 shall be replaced with ASCE 7-22. This is necessary as the 2023 Florida Building Code—Building references ASCE 7-22 and the 2021 International Building Code® references ASCE 7-16/S1.

Use of the Hilti fasteners has also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the Florida Building Code—Building and the Florida Building Code—Residential under the following conditions:

- The use of Hilti Low-velocity Power-actuated Fasteners as a means of attachment for wood blocking, as defined in Section 2330.1.10 of the Florida Building Code—Building, is prohibited.
- The fasteners have not been evaluated for use as cast-in-place anchors for compliance with the High-Velocity Hurricane Zone provisions, and this use is outside the scope of this supplement.



For products falling under Florida Rule 61G20-3, verification that the report holder's quality-assurance program is audited by a quality-assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the evaluation report, reissued September 2023 and revised March 2024.